

NEVADA DEPARTMENT OF TRANSPORTATION
SUPPLEMENTAL NOTICE TO CONTRACTORS NO. 6
Contract 3983

Reference is made to Project NHP-0191(104), on US 395, North of Reno, from McCarran to Golden Valley Structure, in Washoe County, upon which bids will be received until 2:00 p.m., PDT, on the 13th day of July 2023, and opened publicly on that date at the above hour.

Prospective bidders are hereby notified of the following bid item changes:

Deleted Items:

409 0200	PORTLAND CEMENT CONCRETE PAVEMENT (8-INCH)
409 0280	PORTLAND CEMENT CONCRETE PAVEMENT (9 1/2-INCH)

109.06 Partial Payment. This Subsection of the Special Provisions is hereby deleted and the following substituted therefore:

Progress payments will be made biweekly as the work satisfactorily progresses. The progress payments will be based upon material in place, or on the job site and invoiced, and labor expended thereon. The contract price will be adjusted upward or downward according to approved changes throughout the life of the contract. Once 50% of the adjusted contract price has been paid, the Department shall withhold retention in the amount of 5% of said adjusted contract price or \$50,000, whichever is less, until the entire contract is completed satisfactorily and accepted by the Director. The Department may reduce the withheld retention to a minimum of \$10,000 if sufficient reasons exist for reduced retention and written approval has been obtained from every surety furnishing bonds for the work. Any remaining retention shall be withheld until the conditions of Subsection 109.07 Acceptance and Final Payment of the Specifications have been satisfied.

The fifth full paragraph, including subparagraphs (a) and (b), on page 76 of the Standard Specifications is hereby deleted.

The third and fourth sentences of the second to last indented paragraph from the bottom of page 76 of the Standard Specifications are hereby deleted.

The word "original" in two places in the last indented paragraph at the bottom of page 76 of the Standard Specifications is hereby deleted.

The word "original" in five places in subparagraph (c) Mobilization on the middle of page 77 of the Standard Specifications is hereby deleted.

Partial payment will be made on the pay items noted below, provided that materials and equipment are satisfactorily supplied and remain for use on this project only:

CCTV Camera (Fixed)
CCTV Camera (PTZ)
Field Hardened Ethernet Switch

Any materials or equipment requiring testing will receive partial payments of the unit cost according to the following schedule:

1. When successful pre-installation testing of the fiber optic cable is complete and/or upon installation of ITS devices listed above, 70% of the contract unit bid price will be eligible for payment.
2. When successful completion of the SALT and SST is accepted 100% of contract unit bid price will be eligible for payment.

No stockpile payments will be made on the above ITS items of work. All retention is still applicable as detailed in Subsection 109.06.

110.01 Description. The last sentence of page 26 of the Special Provisions is hereby deleted.

409.03.09 Joints. This subsection of the Special Provisions is hereby deleted and the following substituted therefore:

Subparagraph (a) General. Add the following to the end of this Subparagraph of the Standard Specifications:

Submit a Joint Layout Plan for approval a minimum of 20 working days prior to the start of concrete paving operations for all jointing circumstances not shown in the plans. Jointing circumstances not shown in the plans may include but are not limited to entrance and exit ramps and ramp terminals, gore areas, auxiliary lanes and lane tapers, drainage inlets, irregularly shaped panels, structure approaches, and tie-ins to existing PCCP. Determine and show where reinforced panels will be required.

Do not begin concrete paving operations without written approval of the accepted Joint Layout Plan.

Subparagraph (c) Weakened Plane Joints of this Subsection of the Standard Specifications is hereby deleted and the following substituted therefore:

(c) Weakened Plane Joints. Transverse weakened plane joints shall be perpendicular to centerline. Space transverse weakened plane joints according to the plans, except where PCCP is being placed in contact with existing PCCP. Where PCCP is being placed in contact with existing PCCP, transverse joints in the new PCCP shall meet transverse joints in the existing PCCP at the contact joint.

Cut longitudinal weakened plane joints on all lane lines and shoulder lines. Where the combined width of the Portland cement concrete pavement shoulder and adjacent lane is 16 feet or less, omit the longitudinal joint between the shoulder and the lane.

Perform initial 1/8-inch saw cut with a power driven early entry saw designed for sawing fresh concrete without the use of water. Cut joints when saw cutting equipment can be placed on the surface without raveling, tearing, spalling, or causing any other damage to the newly placed concrete pavement. Determine the exact time to saw joints.

After the joint is sawed, clean the saw cut and adjacent concrete surface with a dry vacuum. Immediately revise any procedure used to saw joints which results in uncontrolled random cracking.

Repair portions of curing seal which are disturbed by sawing operations by spraying the areas with additional curing seal.

Keep a standby power saw on the project at all times when concrete paving operations are under way.

The final paragraph on page 190 of the Standard Specifications is hereby deleted and the following substituted therefore:

Dowel bars shall conform to one of the following:

- 1) Solid Dowel Bars. Dowel bars shall be plain, round, smooth, coated bars, free from burrs or other deformations detrimental to free movement of the bars in the concrete. Provide dowel bars of the size and length shown on the plans and with at least one end sawed. Dowel bars shall be Corrosion Resistant Dowel Bars meeting the requirements of AASHTO M254, Type B, except the core material shall be of steel meeting the requirements of AASHTO M31, Grade 300 (40) or Grade 420 (60), or the equivalent, except that the bend test will not be required. The coating material shall meet the coating material requirements of AASHTO M284. Coat the cut ends of the dowel bars. Uniformly apply an approved bond breaker to the coated bar before insertion in the concrete. Use the type of bond breaker as recommended by the coating manufacturer. Submit certified test results showing compliance with all requirements of AASHTO M254 for approval. In particular, see the Pull-Out Test requirement of AASHTO M254. Perform this test on bars to which the proposed bond breaker has been applied. The test report shall identify the type of bond breaker used.
- 2) Tubular Dowel Bars. Tubular dowel bars constructed of a 1.625" outside diameter by 0.120" wall by 18" long tube made from Grade 60 Carbon Steel Tube, produced to ASTM A513 and with a G90 Galvanizing on both the inside and the outside of the tube. The outside diameter will have an additional 9 to 12 mils of ASTM A934 fusion bonded epoxy coating applied. The tubular dowel shall be plugged on both ends using a snug-fitting insert/plug style cap to prohibit any intrusion of concrete or other materials. The outside diameter of the cap should not exceed the outside diameter of the tube, so as to eliminate the possibility of

a joint interlock. Coat dowel bars with bond breaker as described in Solid Dowel Bars.

- 3) Alternative dowel bars. Other dowel bars may be approved by the Engineer upon submission of structural adequacy test results for Pennsylvania Test Method (PTM) 642, showing maximum differential deflection for dynamic and static loading, for both the approach and leave sides of the joint of not more than 0.0075 inches at 1 million cycles, and not greater than a 0.0025 inch increase in differential deflection at 10 million cycles from the corresponding 1 million cycle deflection. The differential deflection is the absolute difference in deflection between the loaded and unloaded sides of the joint. Alternative dowels shall also have equal or better corrosion performance than the solid dowel bars described above. Coat dowel bars with bond breaker as described in Solid Dowel Bars.

502.02.02 Integral Color. This first paragraph of this subsection of the Special Provisions is hereby deleted and the following substituted therefore:

Provide integral color for all special barrier rails, bridge rail, single slope barrier rails, median barrier rail and all rails matching color Dunn Edwards Wooden Peg DE6215. See aesthetic pattering Subsection 212.02.10, 212.03.10, and plans for form liner requirements.

Prospective bidders are hereby directed to remove and destroy plan sheets 3G, 3H, SD7, LS104, and LS105 and replace them with the attached same numbered sheets dated 06/28/2023. Following is a brief explanation of the changes:

Sheet 3G & 3H. Modified area quantity.

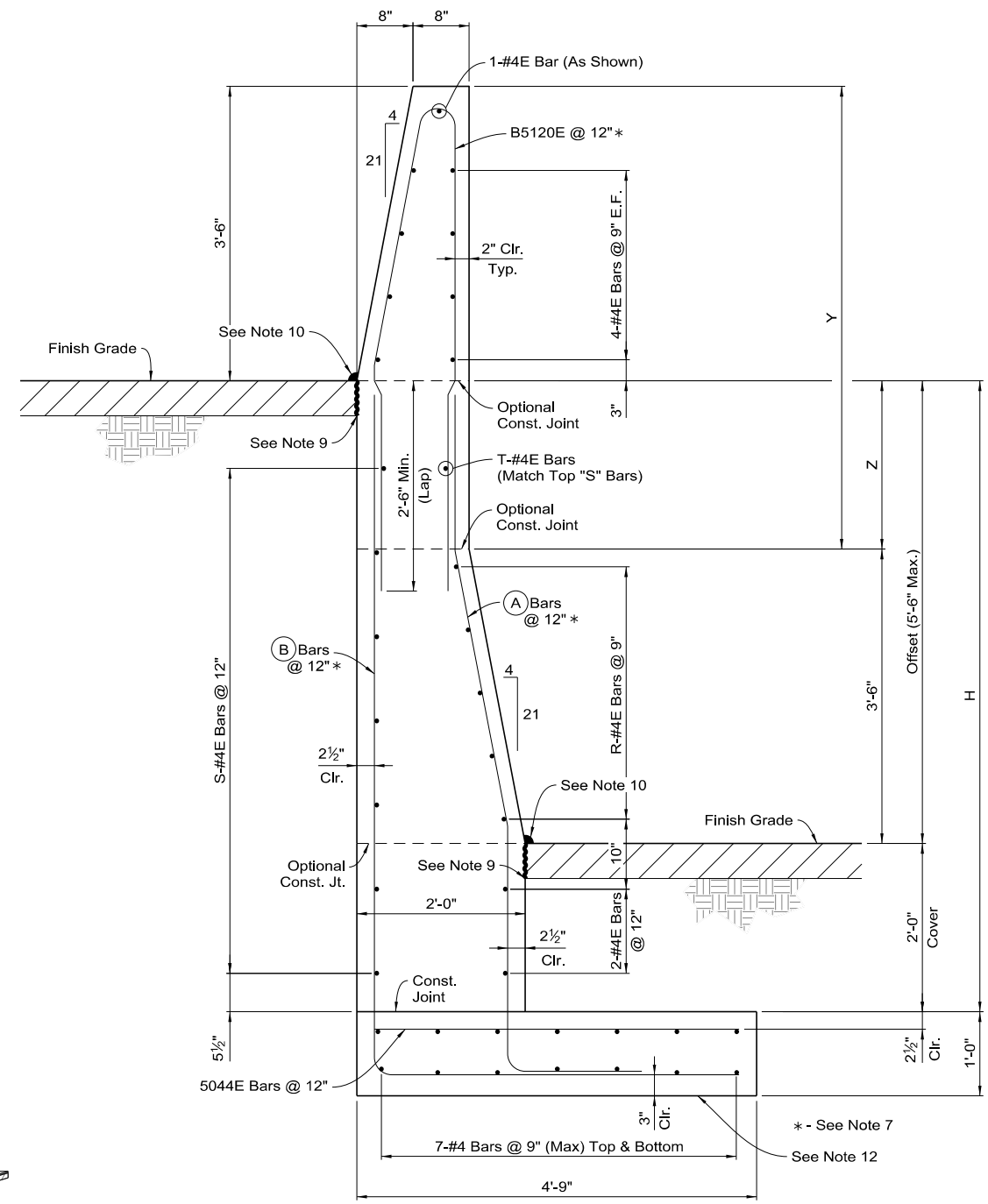
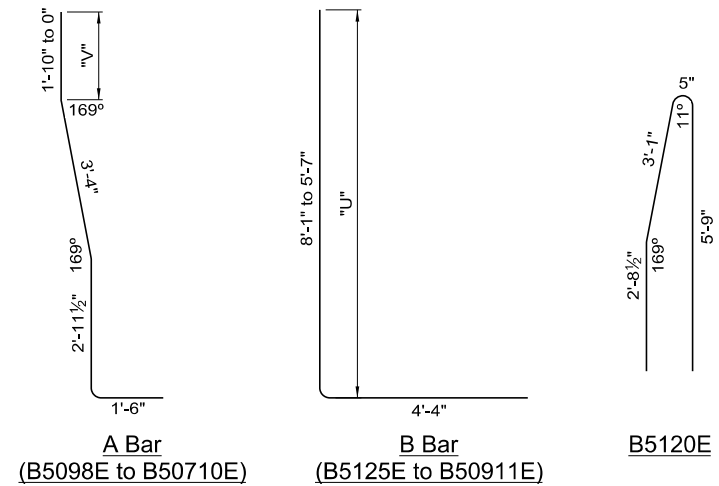
Sheet SD7. Note Deleted.

Sheet LS104 & LS105. Modified bid item.

TRACY LARKIN THOMASON, P.E., DIRECTOR
Nevada Department of Transportation
1263 South Stewart Street
Carson City, Nevada 89712

THE END

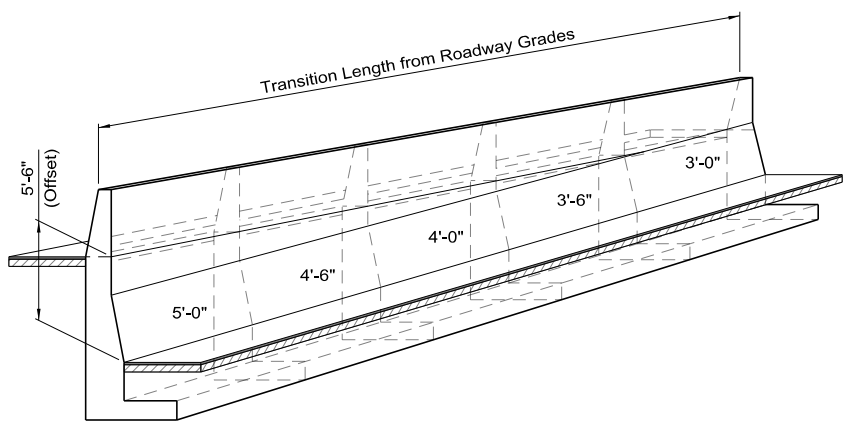
REV. 1 6/28/23 TWK
 NOTE 8 REMOVED.



SPECIAL OFFSET MEDIAN BARRIER WALL DIMENSIONS

Offset	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"
Height "H"	5'-0"	5'-6"	6'-0"	6'-6"	7'-0"	7'-6"
Z	---	0"	6"	1'-0"	1'-6"	2'-0"
Y	3'-0"	3'-6"	4'-0"	4'-6"	5'-0"	5'-6"
Count "R"	4	4	5	5	5	5
Count "S"	5	5	6	6	7	7
Count "T"	---	---	---	---	1	1
A Bar	B50710E	B50710E	B5082E	B5088E	B5092E	B5098E
Dim. "V"	0"	0"	4"	10"	1'-4"	1'-10"
B Bar	B50911E	B5105E	B51011E	B5115E	B51111E	B5125E
Dim. "U"	5'-7"	6'-1"	6'-7"	7'-1"	7'-7"	8'-1"
Concrete Volume (CUFT/FT)	17.42	18.08	18.75	19.42	20.08	20.75
Reinforcing Wt. (LBS/FT)	57.9	58.4	60.7	61.8	64.1	65.2
No. Long. Bars	34	34	36	36	38	38

- GENERAL NOTES**
- Design Specifications: AASHTO LRFD Bridge Design Specifications, 8th Edition, 2017.
 - Loading: Live load surcharge pressure equal to 2'-0" of earth.
 Seismic Acceleration = 0.50g, where 1/2 the peak ground acceleration is used in the design.
 Rail impact loading is TL-4. Inertial effects are assumed to reduce impact load for geotechnical stability limit states to 10 kips. Barrier rail strength limit states are designed using the full TL-4 dynamic impact load.
 - Concrete: All concrete shall be Class AA Modified, Major, with $f_c = 4$ ksi at 28 days.
 - Reinforcing Steel: All reinforcing steel shall be ASTM A615 grade 60 or A706 and shall be epoxy coated unless otherwise noted. Dimensions relating to bars spacing are center-to-center. Bending dimensions are from out-to-out of the bars. Bars marked with a letter prefix indicate a bent bar, with the bar size indicated by the first number of the mark.
 - Design Data: Cantilevered Walls are designed based on the following parameters.
 Soil Properties:
 Internal Angle of Friction = 34°
 Cohesion = 0 ksf
 Unit Weight = 0.120 kcf
 - See Standard Plans CW-5 for requirements for expansion joints, weakened plane joints, and weep holes. Longitudinal reinforcement shall be discontinuous at expansion joints, and consistent with the requirement of CW-5 for weakened plane joints.
 - At wall ends, expansion joint locations and any region where all longitudinal reinforcing is discontinuous, reduce vertical bar spacing to 6-inches (max.) for a minimum distance of 4'-6", measured longitudinally to the wall. These "end" regions shall be installed on both sides of the expansion joint or discontinuity. End regions are not required at weakened plane joints.
 - For excavation and backfill requirements, see Standard Plans EB-4.
 - Vertical joints shall have a single component hot applied sealant full depth of joint.
 - Joint sealer shall be a single component hot applied sealant 1-inch thick.
 - Concrete construction joints designated as a "Permissible Joint" or as an "Optional Construction Joint" may be incorporated into the construction at the Contractor's option. Joints designated as a "Construction Joint" are considered mandatory and shall be incorporated into the construction unless otherwise approved in writing by the Bridge Design Engineer. Roughen construction joints to 1/4" amplitude. Thoroughly clean the surface of all debris and laitance.
 - Bottom of wall footing to follow roadway profile grade.



MINIMUM BAR LAP
 #4E Bar to #4E Bar = 24"
 #5E Bar to #5E Bar = 30"

STATE OF NEVADA
 DEPARTMENT OF TRANSPORTATION

**SPECIAL DETAILS
 MEDIAN OFFSET
 BARRIER WALL**

DATE : 12/12/2022

PCCP SQYD QUANTITIES FOR SLAB REPAIRS REMOVED.

LOCATION	LENGTH STATIONS	BASE AGGREGATE (TYPE 1 CLASS B) INCLUDES SLOPE ALLOWANACE AND 1' WIDENING (includes 8% for moisture)				PORTLAND CEMENT CONCRETE PAVEMENT SURFACE			PLANTMIX DENSE GRADE SURFACE TYPE 2C (WET) INCLUDES ANGLE OF REPOSE				PLANTMIX OPEN GRADE SURFACE 3/8-INCH (WET)			COLD MILLING
		WIDTH FEET	DEPTH INCH	TOTAL VOL. CUYD	UNIT TONS	DEPTH INCH	TOTAL VO CUYD	AREA SQYD	WIDTH FEET	DEPTH INCH	TOTAL VOL CUYD	UNIT TONS	DEPTH INCH	TOTAL VOL CUYD	UNIT TONS	AREA SQYD
CONCRETE SLAB REPAIRS (FOR INFORMATIONAL PURPOSES ONLY)																
"XN" 640+28.00						8.0										
"XN" 642+50.00						8.0										
"XN" 642+61.55						8.0										
"XN1" 785+58.21						9.5										
"XN1" 785+58.45						9.5										
"XN1" 785+73.01						9.5										
"XN1" 785+75.20						9.5										
"XN1" 785+84.35						9.5										
"XN1" 785+84.92						9.5										
"XN1" 795+81.42						9.5										
"XN1" 795+90.81						9.5										
"XN1" 805+10.48						9.5										
"XN1" 805+12.98						9.5										
"XN1" 805+14.74						9.5										
"XN1" 805+20.28						9.5										
"XN1" 805+21.68						9.5										
"XN1" 805+23.85						9.5										
"XN1" 805+34.42						9.5										
"XN1" 805+35.88						9.5										
"XN1" 805+38.86						9.5										
"XN1" 807+94.97						9.5										
"XN1" 807+94.97						9.5										
"XN1" 809+03.62						9.5										
"XN1" 811+76.14						9.5										
"XN1" 812+40.31						9.5										
"XN1" 812+55.33						9.5										
"XN1" 812+71.75						9.5										
"XN1" 812+83.33						9.5										
"XN1" 812+97.92						9.5										
"XS" 619+29.52						8.0										
"XS" 619+45.07						8.0										
"XS" 623+36.23						8.0										
"XS" 625+64.38						8.0										
"XS" 625+75.59						8.0										
"XS1" 799+06.24						9.5										
"XS1" 799+07.26						9.5										
"XS1" 799+19.06						9.5										
"XS1" 799+22.90						9.5										
"XS1" 799+32.95						9.5										
"XS1" 799+37.62						9.5										
"XS1" 799+50.02						9.5										
"XS1" 799+58.63						9.5										
"XS1" 808+17.78						9.5										
"XS1" 809+07.05						9.5										
"XS1" 809+13.83						9.5										
"XS1" 813+08.54						9.5										

CONCRETE SLAB REPAIRS
(FOR INFORMATIONAL PURPOSES ONLY)

1

STATE	PROJECT NO.	COUNTY	SHEET NO.
NEVADA	NHP-0191(104)	WASHOE	LS105

REVISION 6 6/28/23

LANDSCAPE AND AESTHETICS STRUCTURE LIST

623 4100	623 1975	623 1785	623 1780	616 1470	610 0470	610 0190	610 0170	212 1950	212 0930	212 0890	212 0860	212 0840	212 0820	212 0430	212 0410	212 0400	212 0395	212 0045	212 0040	211 0150	211 0100	DESCRIPTION	STATION TO STATION	
EACH	LINFT	LINFT	LINFT	LINFT	CUYD	CUYD	CUYD	EACH	SOYD	SOYD	EACH	EACH	EACH	EACH	EACH	EACH	MONTH	SOYD	SOYD	ACRE	CUYD			
3	7,386	71	102					1.0															INSTALL DECORATIVE STRUCTURE (SE)	"XN" 769+90.00
3	1,878	74	36					1.0															INSTALL DECORATIVE STRUCTURE (NW)	"XS" 771+90.00
																				1.05			20% RESEEDING	
6	9,264	145	138	22,306	11,952	3,548	11,029.5	2	34,654	11,110	13	23	24	27	211	18	12	52,555	28,485	6.38	1,773.9		TOTAL	

